

Reg. No.

**MANIPAL INSTITUTE OF TECHNOLOGY**

MANIPAL

*(A constituent institution of MAHE, Manipal)***VI SEMESTER B.TECH. (CIVIL ENGINEERING)****END SEMESTER EXAMINATIONS, APRIL/MAY 2018****SUBJECT: WASTE WATER MANAGEMENT [CIE 3202]****REVISED CREDIT SYSTEM****(20/ 04/ 2018)**

Time: 3 Hours

MAX. MARKS: 50

Instructions to Candidates:

- ❖ Answer ALL the questions.
- ❖ Missing data may be suitable assumed.

Q.No		Marks	CO
1A.	List and discuss the objectives of biological treatment of wastewater. Explain the unit operation and unit process in detail	5	CO1
1B.	Wastewater sample is being analyzed to determine its BOD content. The sample is diluted to perform the test: 295 mL of distilled water are added to 5 mL of sample to fill the 300 mL BOD bottle. The bottle has an initial dissolved oxygen concentration of 8.2 mg/L. After incubating 5 days, the dissolved oxygen concentration is 4.8 mg/L. i) What is the 5-day BOD of the wastewater? ii) The deoxygenation rate constant, k_d , is 0.13 day^{-1} . What is the ultimate BOD of the wastewater?	3	CO2
1C.	List any 4 advantages and disadvantages of trickling filters used in wastewater treatment	2	CO2
2A.	Design a horizontal grit chamber for population 50000 with water consumption of 1351pcd. (Note: Consider sewage generation 80% of water supply, Maximum flow is 2.5 times the average flow, assume horizontal velocity as 20 cm/sec, Detention time as one minute, 25% additional length to accommodate inlet and outlet zones).	3	CO4
2B.	Assuming suitable design philosophies, design a complete screen chamber to treat a maximum flow of $0.15 \text{ m}^3/\text{s}$ of domestic wastewater. Approach velocity is given as 0.75 m/s. Assume dimensions of bar screen as 10mm x 50mm with spacing of 25mm. The cleaning frequency is once in 3 days and quantity of screenings produced is $0.015 \text{ m}^3/1000 \text{ m}^3$ of waste water per day	5	CO4
2C.	With a diagram briefly explain the Waste Stabilization Pond (WSP) used in wastewater treatment.	2	CO3
3A.	Explain i) Weir Loading ii) Compression settling iii) Detention period in settling tanks	3	CO3
3B.	With a neat sketch explain the working and parts of vertical flow sedimentation tank.	3	CO3

3C.	Determine the size of first and second stage trickling filter of a two stage trickling filter treating a sewage flow of 4MLD at a recirculation ratio of 1.2. The BOD of the incoming sewage to the first stage filter is 300mg/l and BOD of final effluent from secondary filter is 30 mg/l. Efficiency of first stage filter is 76%. Also determine the hydraulic loading and organic loading on the first stage filter. Assume depth of trickling filters as 2m.	4	CO4																				
4A.	<p>Design the activate sludge units for the following data</p> <table><tr><td>Population served</td><td>50000</td></tr><tr><td>Average sewage flow</td><td>180lpcd</td></tr><tr><td>BOD of raw sewage</td><td>200 mg/l</td></tr><tr><td>Raw sewage suspended solids</td><td>300 mg/ l</td></tr><tr><td>BOD removal in primary clarifier</td><td>35%</td></tr><tr><td>Overall BOD reduction desired</td><td>80%</td></tr><tr><td>F/M ratio</td><td>0.2</td></tr><tr><td>MLSS in aeration tank</td><td>3000 mg/ l</td></tr><tr><td>Air requirement</td><td>120 m³/day per kg of BOD removed</td></tr></table> <p>Also check for hydraulic retention time and volumetric loading rate and calculate return sludge ratio assuming SVI as 100.</p>	Population served	50000	Average sewage flow	180lpcd	BOD of raw sewage	200 mg/l	Raw sewage suspended solids	300 mg/ l	BOD removal in primary clarifier	35%	Overall BOD reduction desired	80%	F/M ratio	0.2	MLSS in aeration tank	3000 mg/ l	Air requirement	120 m ³ /day per kg of BOD removed	4	CO4		
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4B.	Mention any 3 merits and demerits of Septic tank.	3	CO3																				
4C.	With a neat sketch differentiate between step aeration and tapered aeration in activated sludge process.	3	CO3																				
5A.	With the flow diagram explain the sludge treatment process.	4	CO3																				
5B.	Mention and explain the zones of pollution in a stream	3	CO4																				
5C.	<p>A wastewater treatment plant disposes off its effluent into a stream at a point A. Characteristics of stream at the upstream of point A and effluent are as follows</p> <table><tr><td>Item</td><td>Units</td><td>Effluent</td><td>Stream</td></tr><tr><td>Flow</td><td>m³/s</td><td>0.2</td><td>0.5</td></tr><tr><td>Dissolved oxygen</td><td>mg/I</td><td>2.0</td><td>8.0</td></tr><tr><td>Temperature</td><td>°c</td><td>26</td><td>22</td></tr><tr><td>BOD₅ at 20°C</td><td>mg/I</td><td>40</td><td>3</td></tr></table> <p>Determine the following after mixing of effluent with the river water at point A. Take deoxygenation constant as 0.1/day and reoxygenation constant 0.3/day and saturation DO as 9.10 mg/l.</p> <ul style="list-style-type: none">i. Combined dischargeii. BODiii. DOiv. Temperaturev. DO at the end of 2 days from the mixing	Item	Units	Effluent	Stream	Flow	m ³ /s	0.2	0.5	Dissolved oxygen	mg/I	2.0	8.0	Temperature	°c	26	22	BOD ₅ at 20°C	mg/I	40	3		CO4
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