



MANIPAL INSTITUTE OF TECHNOLOGY
MANIPAL
(A constituent unit of MAHE, Manipal)

FOURTH SEMESTER B.TECH (CIVIL ENGINEERING)

END SEMESTER EXAMINATION, APRIL-MAY 2024

WASTEWATER MANAGEMENT (CIE 2224)

(05 – 05 - 2024)

TIME: 2:30 – 5:30

MAX. MARKS: 50

- Note: 1. Answer all questions.**
2. Any missing data may be suitably assumed.

Q. NO	QUESTION	MARKS	CO	BL
1A	Explain the basic operation of pneumatic ejectors and airlift pumps with the help of a neat sketch.	05	CO1	2
1B	Explain the difference between conservancy and water carriage system.	03	CO1	2
1C	Illustrate the application of using P and S trap in conveying sewage to a drainage point with a neat sketch.	02	CO1	4
2A	Design a rectangular sedimentation tank for treating the sewage from a city having maximum daily water demand of 2 million liters per day (MLD). Assume a detention time of 2 hours, surface overflow rate (SOR) as $40 \text{ m}^3/\text{m}^2/\text{d}$ and horizontal flow velocity as 0.3 meter/minutes. Assume 85% of water supplied will become sewage. Also, calculate weir loading rate.	05	CO3	3
2B	Explain briefly the three steps involved in Most Probable Number test.	03	CO2	2
2C	Explain various factors affecting the efficiency of Primary sedimentation tank.	02	CO3	2
3A	A single-stage trickling filter is designed for an organic loading of 10,000 kg of BOD in raw sewage per hectare metre per day with a recirculation ratio of 1.5. The filter treats a flow of 2 MLD with BOD concentration in the influent as 300 mg/l. Determine the strength of the effluent. PST removes 25 % of BOD from raw sewage. Assume depth of filter as 2 m.	05	CO4	3
3B	Distinguish between the recirculation process employed in activated sludge process and trickling filters.	03	CO4	4
3C	Explain Sludge Volume Index. How is it determined?	02	CO4	2
4A	Design a conventional activated sludge plant to treat domestic sewage with diffused air aeration system, given the following data: Flow = $2000 \text{ m}^3/\text{d}$; BOD of sewage = 300 mg/l; BOD removed in primary treatment = 30%; Overall BOD reduction = 85%; Assume F/M ratio = 0.4; MLSS concentration = 2500 mg/l; air required per kg of BOD removed = 100 m^3 air/kg BOD; endogenous respiration rate, $k_d = 0.06$; yield coefficient = 0.6.	05	CO4	3

	Also find, HRT, sludge age, rate of air supplied and dimensions of aeration tank if depth and width of 3 m and 4.5 m is to provided respectively.			
4B	Illustrate with the help of a neat sketch the contact stabilization process.	03	CO4	2
4C	Briefly discuss any two methods used for the control of sludge bulking.	02	CO4	2
5A	A town discharges 2000 m ³ /d of sewage into a river having a rate of flow of 2.315 m ³ /s during lean days at a 5-day BOD of sewage and river as 250 and 2 mg/l respectively. Assume the DO of the stream as DO _{sat} and sewage as 1 mg/l. Determine the amount of critical DO deficit and its location in the downstream portion of the river. Assume deoxygenation coefficient K as 0.1, velocity of stream as 0.2 m/s and coefficient of self-purification (fs) as 3.0. Assume saturation DO at given temperature as 9.2 mg/l.	05	CO5	3
5B	Explain the advantages and disadvantages of Rotating Biological Contactor.	03	CO4	2
5C	Explain the factors affecting Self-purification of streams.	02	CO5	2